**Column analysis according to DIN EN 1992-1-1, 5.8.6**

**Materials**

The following conditions shall apply for analysis according to 2nd order theory:

* Geometrical and physical nonlinear analysis of stress resultants, at the limit state of load capacity, as a result of deformation of the structure, including creep- and pre-deformation, Design dimensioning for 1.00-times nonlinear stress resultants
* In the case of percentage of reinforcement up to 2%, the effective stiffness for calculation of required reinforcement is assumed to be a maximum of: E.cm * I.gross * (.2 + 15 As/Ac)
  
  For calculation of the final deformations, however, the effective cross-section values as determined from the strain state are applied without the top-stated limitation. This counters the dangerous risk of an appreciable drop in stiffness for weakly reinforced cross-section in the state 1-->2.
* Material diagram for concrete under normal temperature for eff.EI is according to Fig.3.2 and Eq.3.14, for dimensioning, according to Fig.3.3 and Gl.3.17/3.18, reinforced concrete in all cases according to Fig.3.8
* The verification of constructional fire protection takes place in the simplified, tabular procedure for the fire resistance class R 90.
  
  Exposed on more than one side.
Material under normal temperature

<table>
<thead>
<tr>
<th>Material</th>
<th>Strength</th>
<th>E-Modulus elasticity</th>
<th>Dead load</th>
</tr>
</thead>
<tbody>
<tr>
<td>Concr C 35/45</td>
<td>f.ck = 35.0 N/mm²</td>
<td>E.c0m= 35805. N/mm²</td>
<td>25.0 kN/m³</td>
</tr>
<tr>
<td>Reinfsteel 500</td>
<td>f.yk = 500.0 N/mm²</td>
<td>E.s =200000. N/mm²</td>
<td>78.5 kN/m³</td>
</tr>
<tr>
<td>Prestress.steel</td>
<td>f.pk = 1770. N/mm²</td>
<td>E.p= 195000. N/mm²</td>
<td>78.5 kN/m³</td>
</tr>
</tbody>
</table>

Material diagrams for the following calculations

1. Deformation calculation eff.EI (permanent design situation)
2. Design dimensioning (permanent design situation)
5. Creep under sustained load; Conc. stress = 'Sigma'/(1+creep coef.)

Permissible strains used for loadbearing capacity, or for design dim.:
Concrete on compression -3.5 (o/oo), mean compression force -2.00 (o/oo)
Steel on tension side 10.0 (o/oo)

Partial safety factors used for loadbearing capacity:

<table>
<thead>
<tr>
<th>Analysis</th>
<th>Concrete gamma.c:</th>
<th>Steel gamma.s:</th>
</tr>
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Permissible partial safety factors used for loadbearing capacity:

<table>
<thead>
<tr>
<th>Concrete gamma.c</th>
<th>Steel gamma.s</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.85</td>
<td>1.00</td>
</tr>
</tbody>
</table>

The characteristic curves should be defined at least 1 o/oo more than
the permissible strain. In case of prestressing, the pre-strain constraints
must be taken into account.
Each characteristic curve must contain the point sigma = 0. The following
designation 'quadr' means the middle of a section with quadratic plot.
Building member: Polygonaler Pfeiler - Höhe 41.05 m

Material 1 Concrete for analysis 1 with $k = 1.872$
Strain ($\varepsilon/\varepsilon_0$) -5.00 -2.25 quadr -1.40 quadr -0.60 quadr .0
Sigma (N/mm²) -28.7 -28.7 -27.6 -24.2 -19.3 -12.7 -6.8 0.0

Material 1 Concrete for analysis 2
Strain ($\varepsilon/\varepsilon_0$) -5.00 -2.00 quadr .0
Sigma (N/mm²) -19.8 -19.8 -14.9 0.0

Material 1 Concrete for analysis 5
Strain ($\varepsilon/\varepsilon_0$) -10.00 .0 10.00
Sigma (N/mm²) -358.0 0.0 358.1

Material 2 Reinforcement for analysis 1 2
Strain ($\varepsilon/\varepsilon_0$) -11.00 -2.17 .0 2.17 11.00
Sigma (N/mm²) -434.8 -434.8 0.0 434.8 434.8

Material 2 Reinforcement for analysis 5
Strain ($\varepsilon/\varepsilon_0$) -10.00 .0 10.00
Sigma (N/mm²) -2000.0 0.0 2000.0

System

Length of members

<table>
<thead>
<tr>
<th>Member</th>
<th>Length</th>
<th>begin. Member i</th>
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<tbody>
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<td>30</td>
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<td>1.55</td>
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</tbody>
</table>

Support conditions (valid until new definition)

elast = force proportional and contradirectional to displacement, C positive

Condition in direction C.displacement (kN/m)

C.rotation (kNm/l)
Junc Height $x$ $y$ Phi.$x$ Phi.$y$ C.$x$ C.$y$ C.Phi.$x$
31 0.00 rigid rigid elast elast 0.0 0.0 220208000.0
220208000.0

Imperfection = Pre-deformation (valid until new definition)

Plot = affine to buckling figure
Reference junction = junction 1
Amount = 0.290 m
Direction = determined by program

Dead load $pz$ (kN/m) = 25.0 * A.gross for all load cases

$\gamma.g = 1.35$ Ultimate limit state
Building member: Polygonaler Pfeiler - Höhe 41.05 m

**LCC 1 = permanent load**

kriechzeugende Dauerlasten

<table>
<thead>
<tr>
<th>Height</th>
<th>Px(kN)</th>
<th>Py(kN)</th>
<th>Pz(kN)</th>
<th>Mx(kNm)</th>
<th>My(kNm)</th>
<th>ex(m)</th>
<th>ey(m)</th>
<th>gam</th>
<th>psi</th>
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</tbody>
</table>
Building member: Polygonaler Pfeiler - Höhe 41.05 m

LCC 2 = permanent or transient design situation
Creep due to sustained loading Lc 1 with creep value = 2.00
1.35(G+V)+ 1.50(P+Z+0.8T+S)+1.50x0.6xW

<table>
<thead>
<tr>
<th>Height</th>
<th>Px(kN)</th>
<th>Py(kN)</th>
<th>Pz(kN)</th>
<th>Mx(kNm)</th>
<th>My(kNm)</th>
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<th>psi</th>
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</table>

Line loads (kN/m)

- Memb 3 bis 30: px = 0.00, py = 4.20, pz = 0.00, gam x psi = 0.90
- Memb 1 bis 2: px = 0.00, py = 5.00, pz = 0.00, gam x psi = 0.90

Cross-section 1
distance between reinforcement and edge 0.060 m given

Polygonal cross-section 1

<table>
<thead>
<tr>
<th>y (m)</th>
<th>z (m)</th>
<th>Point</th>
<th>Concrete= Material 1</th>
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Section 1  1Local
Building member: Polygonaler Pfeiler - Höhe 41.05 m

Cross-section 2
distance between reinforcement and edge 0.060 m given

Polygonal cross-section 1

<table>
<thead>
<tr>
<th>y (m)</th>
<th>z (m)</th>
<th>Point</th>
<th>Concrete= Material 1</th>
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<tbody>
<tr>
<td>0.000</td>
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Point-, line- und ring-reinforcement

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<th>Prio</th>
<th>Mat-no</th>
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<th>Point 2</th>
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<td>z1(m)</td>
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<td>-1.350</td>
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</table>

Section 2  1Local

Cross-section 3
distance between reinforcement and edge 0.060 m given

Polygonal cross-section 1

<table>
<thead>
<tr>
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<th>z (m)</th>
<th>Point</th>
<th>Concrete= Material 1</th>
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</thead>
<tbody>
<tr>
<td>0.000</td>
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</table>
Building member: Polygonaler Pfeiler - Höhe 41.05 m

-1.680 -1.410 10

Point-, line- und ring-reinforcement

| No | Slap | Prio | Mat-no | Reinforcement As | Point 1 | Point 2 | reflec-
<table>
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<tr>
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<td>2.370</td>
<td>-0.850</td>
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<tr>
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<td>Line</td>
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<td>2</td>
<td>0.01000.0 cm²/m</td>
<td>0.01000.0 cm²/m</td>
<td>-2.370</td>
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<tr>
<td>4</td>
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<td>2</td>
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Section 3 1 Local

Cross-section 4
distance between reinforcement and edge 0.090 m given

Polygonal cross-section

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Point-, line- und ring-reinforcement

| No | Slap | Prio | Mat-no | Reinforcement As | Point 1 | Point 2 | reflec-
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Section 4 3 Sections
Building member: Polygonaler Pfeiler – Höhe 41.05 m

**Cross-section 5**

distance between reinforcement and edge 0.090 m given

**Polygonal cross-section**

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Point-, line- und ring-reinforcement

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**Cross-section 6**

distance between reinforcement and edge 0.090 m given

**Polygonal cross-section**

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Building member: Polygonaler Pfeiler - Höhe 41.05 m

Point-, line- und ring-reinforcement

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2. Line 1 2 0.01 000.0 cm²/m -1.000 0.990 1.000 0.990
3. Line 1 2 0.01 000.0 cm²/m -1.750 0.570 -1.750 -0.570
4. Line 1 2 0.01 000.0 cm²/m 1.750 -0.570 1.750 0.570
5. Line 1 2 0.01 000.0 cm²/m -1.750 -0.570 -1.000 -0.990
6. Line 1 2 0.01 000.0 cm²/m 1.000 0.990 1.750 0.570
7. Line 1 2 0.01 000.0 cm²/m -1.750 0.570 -1.000 0.990
8. Line 1 2 0.01 000.0 cm²/m 1.750 0.570 1.000 0.990

Section 6 2 Sections

Cross-section 7
distance between reinforcement and edge 0.120 m given

Polygonal cross-section

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Point-, line- und ring-reinforcement

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1. Line 1 2 0.01 000.0 cm²/m -1.000 -0.990 1.000 -0.990
2. Line 1 2 0.01 000.0 cm²/m -1.000 0.990 1.000 0.990
3. Line 1 2 0.01 000.0 cm²/m -1.750 0.570 -1.750 -0.570
4. Line 1 2 0.01 000.0 cm²/m 1.750 -0.570 1.750 0.570
5. Line 1 2 0.01 000.0 cm²/m -1.750 -0.570 -1.000 -0.990
6. Line 1 2 0.01 000.0 cm²/m 1.000 0.990 1.750 0.570
7. Line 1 2 0.01 000.0 cm²/m -1.750 0.570 -1.000 0.990
8. Line 1 2 0.01 000.0 cm²/m 1.750 0.570 1.000 0.990

Section 7 4 Sections
Building member: Polygonaler Pfeiler - Höhe 41.05 m

Cross-section 8

Polygonal cross-section

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Point-, line- und ring-reinforcement

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Cross-section 9

Polygonal cross-section

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Building member: Polygonaler Pfeiler - Höhe 41.05 m

Point-, line- und ring-reinforcement

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Section 9  45Sections

Cross-section 10
distance between reinforcement and edge 0.120 m given

Polygonal cross-section

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Point-, line- und ring-reinforcement

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Section 10  45Sections
Building member: Polygonaler Pfeiler - Höhe 41.05 m

Cross-section 11

distance between reinforcement and edge 0.120 m given

Polygonal cross-section 1

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Point-, line- und ring-reinforcement

No Slap Prio Mat-no  Reinforcement As Point 1 Point 2 refle-
min max y1(m) z1(m) y2(m) z2(m) cted
1 Line 1 2 0.01000.0 cm²/m -1.200 -1.120 1.200 -1.120 |
2 Line 1 2 0.01000.0 cm²/m -1.200 1.120 1.200 1.120 |
3 Line 1 2 0.01000.0 cm²/m -1.900 0.740 -1.900 -0.740 |
4 Line 1 2 0.01000.0 cm²/m 1.900 -0.740 1.900 0.740 |
5 Line 1 2 0.01000.0 cm²/m -1.900 -0.740 -1.150 -1.120 |
6 Line 1 2 0.01000.0 cm²/m 1.150 1.120 1.900 0.740 |
7 Line 1 2 0.01000.0 cm²/m -1.900 0.740 -1.150 1.120 |
8 Line 1 2 0.01000.0 cm²/m 1.900 0.740 1.150 1.120 |

Section 11 4Sections

Cross-section 12

distance between reinforcement and edge 0.120 m given

Polygonal cross-section 1

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Building member: Polygonaler Pfeiler - Höhe 41.05 m

Point-, line- und ring-reinforcement

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Section 12  4Sections

Cross-section 13

distance between reinforcement and edge 0.120 m given

Polygonal cross-section

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Point-, line- und ring-reinforcement

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Section 13  4Sections
Building member: Polygonaler Pfeiler - Höhe 41.05 m

Cross-section 14

distance between reinforcement and edge 0.120 m given

Polygonal cross-section

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Point-, line- und ring-reinforcement

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Section 14  4Sections

Cross-section 15

distance between reinforcement and edge 0.120 m given

Polygonal cross-section

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Building member: Polygonaler Pfeiler - Höhe 41.05 m

Point-, line- und ring-reinforcement

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Section 15    4 Sections

Cross-section 16

distance between reinforcement and edge 0.120 m given

Polygonal cross-section

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Point-, line- und ring-reinforcement

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Section 16    4 Sections
Building member: Polygonaler Pfeiler - Höhe 41.05 m

**Cross-section 17**

distance between reinforcement and edge 0.120 m given

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Point-, line- und ring-reinforcement

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**Cross-section 18**

distance between reinforcement and edge 0.120 m given

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Building member: Polygonaler Pfeiler - Höhe 41.05 m

Point-, line- und ring-reinforcement

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Cross-section 19

distance between reinforcement and edge 0.120 m given

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Section 18  5Sections

Section 19  1Local
Results

LCC 2 1st order theory, service loads

\[1.35(G+V)+1.50(P+Z+0.8T+S)+1.50 \times 0.6xW\]

LCC 2 1st order theory, design action for minimum reinforcement

\[1.35(G+V)+1.50(P+Z+0.8T+S)+1.50 \times 0.6xW\]

Creep deformations under sustained loading LCC 1 with creep value = 2.00

Material diagrams for analysis no. 5

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## LCC 2 Design resistance analysis (2nd order theory, Basic combination)

**Material diagrams for analysis no. 1 2**

\[
1.35(G+V) + 1.50(P+Z+0.8T+S)+1.50\times0.6\times W
\]

## Effective cross-section values

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## Building member: Polygonaler Pfeiler - Höhe 41.05 m

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Building member: Polygonaler Pfeiler – Höhe 41.05 m

Foundation loads [Type: 1 = 1.00-fold, 2 = gamma-fold]

1.00-times foundation loads apply for geo-static verification and gamma-times foundation loads apply for design dimensioning for the following building members.

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Required reinforcement

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Building member: Polygonaler Pfeiler - Höhe 41.05 m

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Total: 13485.6 cm²/m²

As/A.gross = 0.16%

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Total: 9059.0 cm²/m²

As/A.gross = 0.17%

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Total: 9718.4 cm²/m²

As/A.gross = 0.45%
### Building member: Polygonaler Pfeiler - Höhe 41.05 m

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Total
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1.0 10781.4 569.5 Section 10 As/A.gross = 0.63 %

1.0 11123.1 614.0 Section 11 As/A.gross = 0.64 %

1.0 11478.8 641.0 Section 12 As/A.gross = 0.64 %
Building member: Polygonaler Pfeiler - Höhe 41.05 m

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Total: 0.0 12065.2 662.5 Section 14 As/A.gross = 0.60 %

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Total: 0.0 12363.1 652.1 Section 15 As/A.gross = 0.56 %

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Total: 0.0 13043.1 588.2 Section 17 As/A.gross = 0.45 %
Building member: Polygonaler Pfeiler - Höhe 41.05 m

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Total: 0.0 13377.1 559.9 Section 18  As/A_gross = 0.42%

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Height to(m)  Lateral Section d1(m)  As(cm²)  As/A_gross

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Total long. reinforcement = 14475 kg (without anchorage length etc.)
Result graphics
RIB BEST 13.0   (c)2013 RIB Software AG

Longitudinal reinforcement [cm²]

max 653.80  min 187.50
LC2: 1.35(G+V) + 1.50(P+Z+0.8T+S)...
Normal force, 2nd order theory [kN]
max -28687.20 min -42268.60

LC2: 1.35(G+V) + 1.50(P+Z+0.8T+S)...
Bending moments 2nd order theory [kNm]
max -152.90 min -34119.20
Shear forces, 2nd order theory [kN]

max 51.60  min -497.90

Displacements 1.0-fold [mm]

max 35.51  min -6.92
LC2: 1.35(G+V) + 1.50(P+Z+0.8T+S)...

Displacements, 2nd order theory [mm]
max 513.65 min -62.15

LC2: 1.35(G+V) + 1.50(P+Z+0.8T+S)...

Initial displacements, 2nd order theory [mm]
max 331.11 min -40.04